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CIVIL & STRUCTURAL ENGINEERING

Do Not Scale.

in plotting size

reduction/increase

Check for

ALL DIMENSIONS ARE TO BE CHECKED

ON SITE BEFORE COMMENCING AND AT

ALL STAGES OF CONSTRUCTION

NOTES

The pump station and all related valve chambers and facilities shall comply with the detailed requirements of "Part 5" Pump Stations" of Irish Water's Code of Practice for Wastewater.

Some particular requirements are set out below and on this drawing.

A dedicated, metered, power supply to be provided to the pump station serving only the pump station equipment and associated plant;

Adequate site security lighting is provided to achieve 100 lux at ground level, with intensity adjustment appropriate for the site location, complete with photoelectric cell controller and over-ride control switch. Wash hand basin to be provided wet

Detailed specification for the Boundary Fencing and gates to be as outlined in Section 5.6 of the Code

Alert system and call out emergency response to be provided in the event of plant breakdown or malfunction;
The plant shall be provided with a telemetry outstation to transfer data from the pumping station to an Irish Water control centre. The data to be transferred to Irish Water Control Centre shall include at least the following:

5.8.1 Available/Run/Trip status for all pumps; 5.8.2 Status for all float switches;

5.8.3 Sump level; 5.8.4 Instantaneous flow;

5.8.5 Totalised flow; 5.8.6 Mains Power Failure; 5.8.7 UPS Fault/Healthy Status.

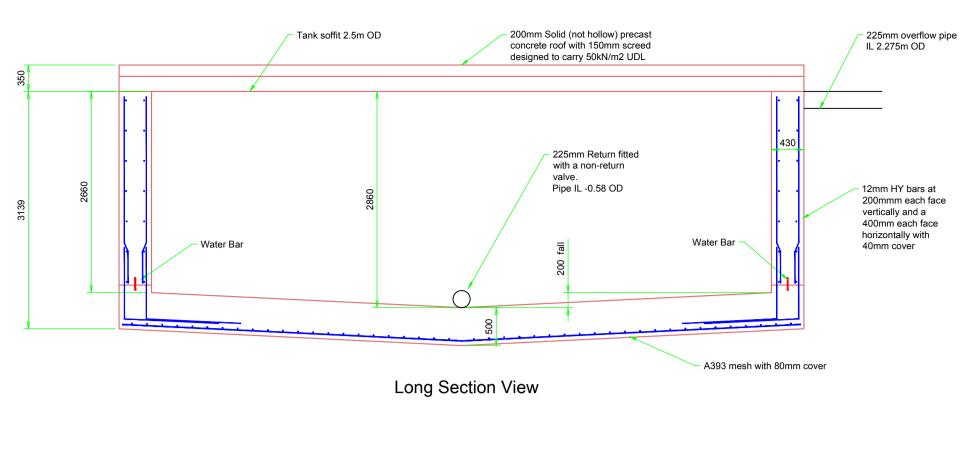
Surge analysis should be carried out and appropriate counter measures employed if necessary.

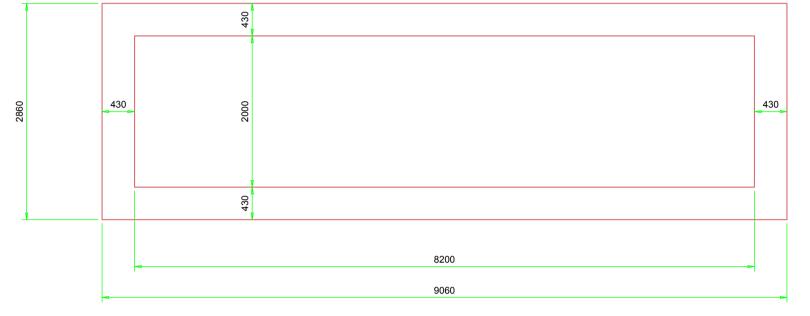
A hard wired low level float switch and high level float switch should be provided in each pumping station and these should be linked to the telemetry control

Provision should be made for isolating the incoming flow by means of a hand-operated valve or penstock. This unit should be located in a chamber upstream of the wet well and not in the wet well itself.

A 225mm diameter high-level overflow pipe, or a pipe to match the capacity of the incoming Sewer, shall be provided between the pump station wet well and the storage chamber if off line storage is provided. The return pipe feeding back to the pump station shall be a minimum of 225mm diameter and shall be fitted with a proprietary non-return valve at the wet well chamber.

Flow meters shall be provided to measure and record the Wastewater flow being pumped forward through the Rising Main. Magnetic flow meters shall be provided with flow recorders linked to converters in the MCC panel of the control kiosk, complete with a digital display showing instantaneous and accumulated flow records





Plan View
Emergency Storage Tank

A 225mm diameter high-level overflow pipe, or a pipe to match the capacity of the incoming Sewer, shall be provided between the pump station wet well and the storage chamber if off line storage is provided. The return pipe feeding back to the pump station shall be a minimum of 225mm diameter and shall be fitted with a proprietary non-return valve at the wet well chamber.

First Issue Date

Nov 2018

PL 08

Civil Engineering Drawings

SEWAGE PUMP STATION

AM

1:50 and 1:100 on A²

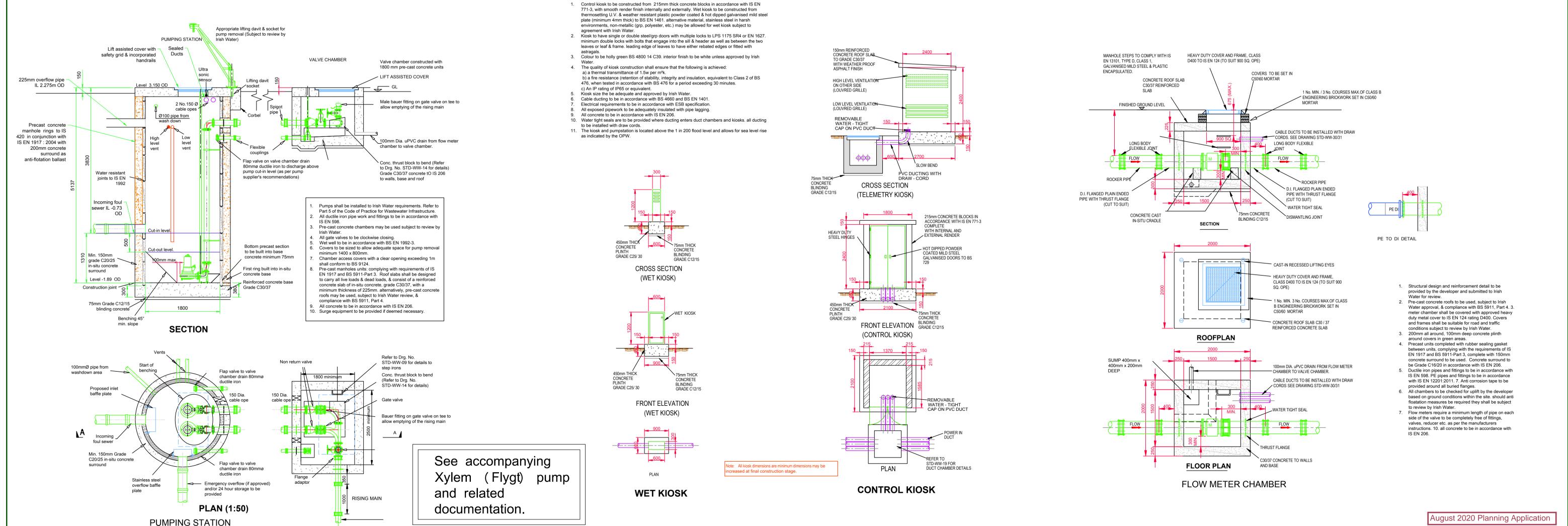
Planning

RESIDENTIAL DEVELOPMENT

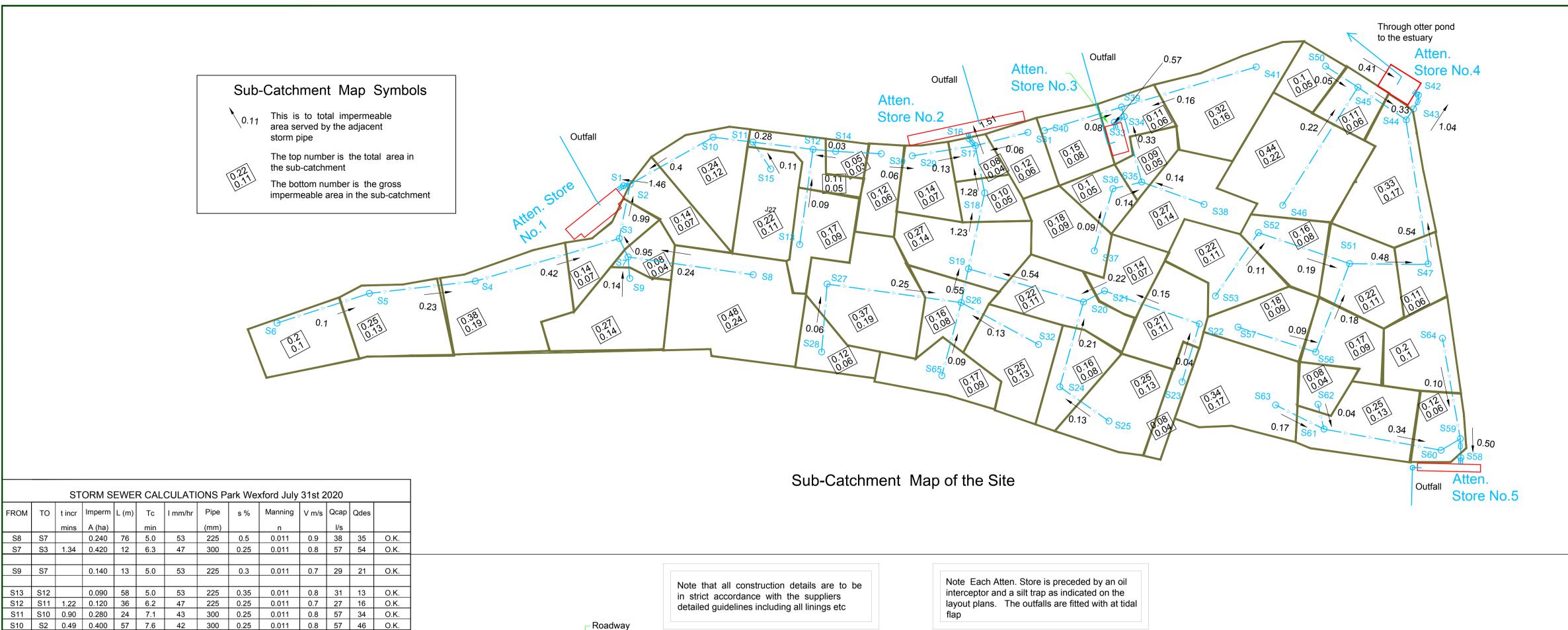
PARK

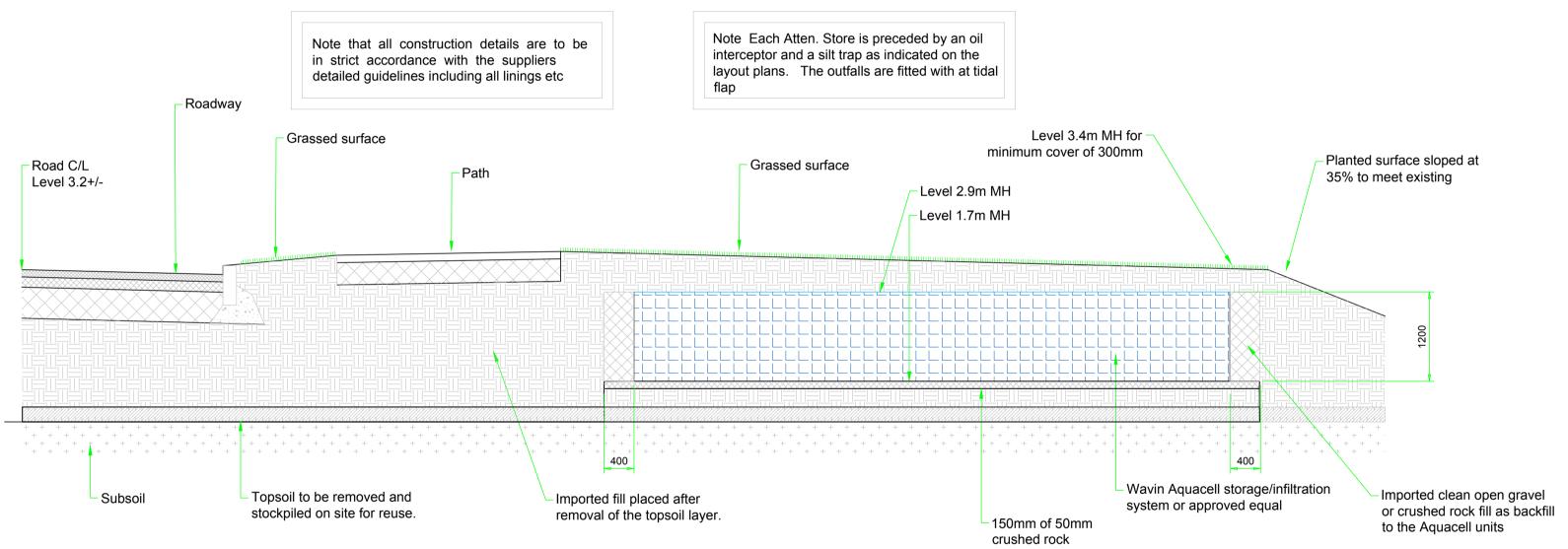
WEXFORD

Date



No Revision Description





Typical Cross Section through Attenuation Store (Scale 1:50)

Storm Water System.

Wexford County Council requires attenuation facilities for all storm water, up to the 100 year design storm, before discharge to Wexford Harbour/Estuary. A standard storm water collection system is proposed with 5 attenuation storage facilities designed to this requirement. The permitted discharge is calculated based on the recommendations of the Greater Dublin Strategic Drainage Study.

The discharge pipes discharge to the estuary and are buried 500mm under the shore with concrete protection to below the low tide mark. Each outfall is to be fitted with a non-return tidal flap.

Each attenuation store is preceded by an oil interceptor and a silt trap as indicated on the layout plans.

Concrete attenuation tanks of the same depth and with 95% of the clear floor area required for the Aquacell system may be used if this should be preferred by Planning.

Summary of A	Attenua	ation D	etails		
Gallery Number	No. 1	No. 2	No. 3	No. 4	No.
Total Area (ha)	2.746	2.946	1.22	1.85	1.
Percentage impermeable area (%)	55	55	55	55	
Impermeable Area (ha)	1.5103	1.6203	0.671	1.0175	0.64
% of Imp.area contributing directly to the drainage system (Dublin GSS)	75	75	75	75	
Impermeable area directly contributing to the Gallery (ha)	1.13	1.22	0.50	0.76	0.
Permitted Outflow (litres per second)	16.9	18.2	7.5	11.4	•
Attenuation Storage required	454	487	202	306	1
Depth of Aquacell array	1.2	1.2	1.2	1.2	,
Plan Area of Aquacell system	398	427	177	268	1
Aquacell array length	39.0	47.5	18.0	20.0	34
Average width	10.2	9.0	10.0	13.5	;
Attenuation Storage provided	454	487	205	308	1

SAMPLE STORM WATER ATTENUATION CALCULATIONS

The calculations are based on the Greater Dublin Strategic Drainage Study.

The attenuation storage is to be provided in an attenuation tank as detailed here and the discharge is to be limited, by means of a Hydrobrake or approved alternative, to the calculated allowable discharge set out

Extreme Rainfall Return Periods (Source - Met Eireann)

Location	•		WEXFC	(FORD						
110 % of	f Maxin	num rai	nfall (mr	n) of indi	cated du	ration for t	he indic	ated retu	rn perio	
				Return P	eriod (ye	ars)	•			
Duration				5	10	20	50	100		
60 min				19	23	27	34	40		
2 hour				24	29	34	43	50		
4 hour				31	37	43	53	62		
6 hour				36	42	49	60	70		
12 hour				46	54	62	75	86		

Site Details

CALCULATION OF GREEN FIELD RUNOFF

Impermeable area contributing to the the drainage system

24 hour

Given a si	te area of			1.22	hectares				
Area for ca	alculation	of AREA		50	ha (recom	mended mini	mum in G	reater Dub	in Study)
AREA in k	m2			0.5	km ²		(SAAR)	1163	mm
SOIL				0.4	for Silty (I	ntermediate)	soils		
Soil type a	t the site	is aluvial	Silt (Soil M	lap of Co. V	Vexford (N	ational Soil S	urvey of Ir	eland))	
QBAR rura	al for this	AREA		0.31	m3/s	= 0.00108	* AREA O.	89 * SAAR	17 * Soil
QBAR per	hectare			6.16	l/s				
Permissab	le outflov	v will then	be					7.5	l/s
Impermea	ble area							0.67	ha
% of impe	rmeable a	rea contr	ibuting to	direct runof	to the drai	nage system		75	%
(Per Appe	ndix E-2	Greater D	ublin Strat	egic Draina	ge Study)		_		

RUNOFF VOLUME

The runoff volume, in cubic metres, from the catchment for all the storms listed in the rainfall table

0.50 ha

storage for

above is set out below:

	Return Period (years)								
Duration				5	10	20	50	100	
60 min				95	115	137	172	203	
2 hour				122	146	173	214	251	
4 hour				156	185	218	266	310	
6 hour				179	213	249	303	351	
12 hour				229	270	313	378	434	
24 hour				293	342	393	470	536	

The allowable outflow and required storage for various durations are:

							100 yr sto	rm
60 min	60	mins			27	m3	176	m3
2 hour	120	mins			54	m3	197	m3
4 hour	240	mins			108	m3	202	m3
6 hour	360	mins			162	m3	189	m3
12 hour	720	mins			325	m3	109	m3
24 hour	1440	mins			650	m3	0	m3
The stora	ge requ	ired on s	site for a	100 year s	torm wou	ıld be	202	m3

allowable butflow

Aquacell Volume Calculations are as follows

Gross Storage allowing for voids	212	m3	Soffit level of tank	2.9	m OD
Proposed Aquacell invert level	1.7	m OD			
Internal operational level	1.2	m OD			
Floor area of Aquacells	177	m2			
Nett Storage provided	202	m3			

Alternative Tank Volume Calculations are as follows

Volume of storage	202	m3	Soffit level of tank	2.9	т
Proposed invert level of tank	1.7	m OD	Depth of precast roof units	200	mm
Internal operational level	1.2	m OD			
Tank internal area ex. all walls	168	m2	Top of concrete	3.10	m C
Storage provided	202	m3			

August 2020 Planning Application

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Do Not Scale. Check for reduction/increase in plotting size

0.110 20 5.0 53 225 0.3 0.011 0.7 29 16 O.K.

0.030 14 5.0 53 150 0.3 0.011 0.6 10 4 O.K.

0.130 37 5.0 53 225 1.14 0.011 1.4 57 19 O.K.

0.130 37 5.0 53 225 0.3 0.011 0.7 29 19 O.K.

0.060 32 5.0 53 300 0.5 0.011 1.1 81 9 O.K.

0.140 40 5.0 53 225 0.25 0.011 0.7 27 21 O.K.

0.160 48 5.0 53 300 0.25 0.011 0.8 57 24 O.K.

0.080 84 5.0 53 300 0.25 0.011 0.8 57 12 O.K.

0.090 49 5.0 53 150 1.57 0.011 1.3 23 13 O.K.

0.170 33 5.0 53 225 1.56 0.011 1.7 66 25 O.K.

0.100 61 5.0 53 150 2.07 0.011 1.5 26 15 O.K.

0.10 58 6.0 48 150 1.02 0.011 1.0 18 13 O.K.

0.04 36 5.0 53 150 5.31 0.011 2.4 42 6 O.K.

0.09 39 5.0 53 150 2.16 0.011 1.5 27 13 O.K.

0.22 | 17 | 5.0 | 53 | 225 | 0.48 | 0.011 | 0.9 | 37 | 33 | O.K.

0.11 46 5.0 53 150 1.6 0.011 1.3 23 16 O.K.

0.050 | 24 | 5.0 | 53 | 150 | 0.5 | 0.011 | 0.7 | 13 | 7

 S24
 S20
 0.43
 0.210
 54
 5.4
 51
 225
 1.14
 0.011
 1.4
 57
 30
 O.K.

S27 | S26 | 0.45 | 0.250 | 81 | 5.4 | 51 | 225 | 1.15 | 0.011 | 1.4 | 57 | 35 | O.K. S26 S19 0.94 0.550 22 6.4 46 300 0.4 0.011 1.0 72 71 O.K.

 S29
 S17
 0.84
 0.210
 54
 5.8
 49
 225
 0.3
 0.011
 0.7
 29
 28
 O.K.

S39 S34 1.73 0.240 6 6.7 45 300 0.3 0.011 0.9 63 30 O.K.

S56 S51 0.64 0.180 57 5.6 50 225 0.96 0.011 1.3 52 25 O.K.

S5 S4 0.94 0.23 64 6.9 44 225 0.52 0.011 1.0 38 28 O.K.
 S4
 S3
 1.11
 0.42
 90
 8.0
 40
 300
 0.99
 0.011
 1.6
 114
 47
 O.K.
 S3 S2 0.98 0.99 33 9.0 38 375 0.38 0.011 1.2 128 104 O.K.

S2 S1 0.47 1.46 7 9.5 37 450 0.38 0.011 1.3 208 149 O.K.

S22 S21 0.26 0.15 60 5.3 52 225 1.14 0.011 1.4 57 22 O.K.

S21 S20 0.70 0.22 14 6.0 48 225 2.84 0.011 2.3 90 29 O.K.

S20 S19 0.10 0.54 72 6.1 48 300 0.91 0.011 1.5 109 72 O.K.
 S19
 S18
 0.78
 1.23
 46
 6.8
 45
 450
 0.33
 0.011
 1.2
 194
 152
 O.K.

S18 S17 0.63 1.28 29 7.5 42 450 0.33 0.011 1.2 194 150 O.K.

S17 S16 0.40 1.51 7 7.9 41 450 0.3 0.011 1.2 185 172 O.K.

S36 S35 0.43 0.14 18 5.4 51 225 0.2 0.011 0.6 24 20 O.K.

S35 S34 0.50 0.33 41 5.9 48 300 0.2 0.011 0.7 51 44 O.K.

S34 S33 0.94 0.57 7 6.9 44 300 0.5 0.011 1.1 81 70 O.K.

 S45
 S44
 0.31
 0.33
 35
 5.3
 51
 300
 0.25
 0.011
 0.8
 57
 47
 O.K.

S52 S51 0.59 0.19 58 5.6 50 225 0.4 0.011 0.8 34 26 O.K.

S51 S47 1.14 0.48 42 6.7 45 375 0.15 0.011 0.7 80 60 O.K.
 S47
 S44
 0.96
 0.54
 7
 7.7
 42
 375
 0.15
 0.011
 0.7
 80
 62
 O.K.

 S44
 S42
 0.16
 1.04
 7
 7.9
 41
 450
 0.15
 0.011
 0.8
 131
 119
 O.K.

 S62
 S61
 0.04
 15
 5.0
 53
 150
 1.43
 0.011
 1.2
 22
 6
 O.K.

 S61
 S60
 0.20
 0.34
 71
 5.2
 52
 300
 0.5
 0.011
 1.1
 81
 49
 O.K.

 S60
 S59
 1.03
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 15
 6.2
 47
 300
 0.5
 0.011
 1.1
 81
 52
 O.K.

 S59
 S58
 0.22
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 7
 6.5
 46
 300
 0.99
 0.011
 1.6
 114
 64
 O.K.

S32 | S26 | 0.090 | 45 | 5.0 | 53 | 150 | 2.03 | 0.011 | 1.5 | 26 | 13

0.060 42 5.0 53 150 2.34 0.011 1.6 28 9

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No Revision Description Date

RESIDENTIAL DEVELOPMENT **WEXFORD**

Sub Project Civil Engineering Drawings STORM SYSTEM DESIGN

First Issue Date AM NTS Dec 14, 2019 Drawing No. Revision PL 09 Planning

